

1. THE CONTRACTOR AND ALL SUBCONTRACTORS SHALL CONFORM TO THE REQUIREMENTS OF "INTERNATIONAL BUILDING CODE - 2018", AND ALL REFERENCED CODES INCLUDED THEREIN.
2. EXAMINE ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL & PLUMBING DRAWINGS FOR VERIFICATION OF LOCATION AND DIMENSIONS OF ITEMS.
3. EACH CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL COMPONENTS OF THE WORK, INCLUDING DESCRIPTION OF CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE DESIGN PROFESSIONAL.
4. THE COMPLETED BUILDING SHALL BE HANDICAP ACCESSIBLE AND SHALL CONFORM TO THE "ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES - ICC/ANSI A117.1-2009".
5. EACH CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS, TOLERANCES, CONSTRUCTION CONDITIONS, ETC. AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO THE COMMENCEMENT OF ANY WORK.
6. ALL DIMENSIONS GIVEN TO FACE OF FRAMING, FACE OF CONCRETE, OR CENTERLINE OF STEEL, UNLESS NOTED OTHERWISE.
7. THE GENERAL CONTRACTOR SHALL COORDINATE WITH STEEL COLUMN MANUFACTURER FOR DELIVERY AND INSTALLATION OF THE STRUCTURAL STEEL FRAME.
8. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING A PA ONE-CALL.
9. THE TOWNSHIP RESERVES THE RIGHT TO ELIMINATE ANY PORTION OF THE WORK AT ANYTIME.

1. DESIGN GRAVITY LOADS:
  - ROOF DEAD LOAD = 10 PSF
  - SNOW LOAD = 30 PSF
  - COLLATERAL LOAD = 5 PSF
  - ROOF LIVE LOAD = 20 PSF
  - FLOOR LIVE LOAD = 50 PSF (OFFICES)
  - FLOOR LIVE LOAD = 125 PSF (LIGHT STORAGE)
2. BASIC WIND SPEED = 115 MPH
3. WIND EXPOSURE WIND EXPOSURE = "B"
4. WIND IMPORTANCE FACTOR = (I) 1.0
5. EARTHQUAKE LOADS = PER IBC – 2018

1. FOOTINGS WHICH WILL BE EXPOSED TO FROST PENETRATION SHOULD BE A MINIMUM OF 36 INCHES BELOW FINISH GRADE.
2. FOOTINGS WHICH WILL BEAR IN UNDISTURBED RESIDUUM SHOULD BE POSITIONED NOT LESS THAN 18 INCHES BELOW EXISTING GROUND SURFACE ELEVATIONS, UNLESS APPROVED BY GEOTECHNICAL ENGINEER.
3. WHEN BEDROCK IS ENCOUNTERED AT FOUNDATION BEARING ELEVATION, IT SHOULD BE REMOVED TO A LEVEL THAT IS TWO FEET BELOW BEARING. FOUNDATION BEARING ELEVATION SHOULD SUBSEQUENTLY BE RE-ESTABLISHED WITH STRUCTURAL FILL MATERIAL. IN THE HORIZONTAL DIRECTION, THE ROCK SHOULD BE REMOVED NOT LESS THAN 6 INCHES BEYOND THE SIDES OF THE FOUNDATIONS. FOUNDATION CONSTRUCTION SHOULD NOT PERMIT THE SIDES OF FOOTINGS TO MAKE CONTACT WITH THE BEDROCK. ALL FOOTING SUBGRADES SHOULD BE PROBED TO ENSURE AT LEAST 15 INCHES OF CONSOLIDATED STRATA IS POSITIONED BELOW BEARING ELEVATION.
4. IT SHOULD BE ANTICIPATED THAT THE BEARING CAPACITY OF THE EXISTING UNDISTURBED OVERBURDEN AT CERTAIN LOCATIONS WITHIN THE FOOTPRINT OF THE ADDITION STRUCTURE WILL NOT HAVE THE DESIGN STRENGTH OF 2,500 P.S.F. THEREFORE SOME REHABILITATION OF THE FOUNDATION SUBGRADE MAY BE REQUIRED IN ORDER TO REALIZE SUCH A BEARING VALUE.
5. ALL BACKFILL PLACED WITHIN THE FOOTPRINT OF THE PROPOSED ADDITION SHOULD BE COMPACTED TO AT LEAST 95% OF THE MAXIMUM DRY DENSITY WITHIN TWO (2) PERCENT, PLUS OR MINUS, OF THE OPTIMUM MOISTURE CONTENT DETERMINED BY THE STANDARD COMPACTION TEST, ASTM DESIGNATION D 698.
6. ALL BACKFILL SHOULD BE PLACED IN LOOSE LIFTS NOT EXCEEDING EIGHT (8) INCHES IN THICKNESS. WHERE HANDHELD EQUIPMENT SUCH AS "WACKER" TYPE TAMPERS AND "WALK BEHIND" ROLLERS ARE EMPLOYED FOR COMPACTION, THE LOOSE LIFT THICKNESS SHOULD BE REDUCED TO A MAXIMUM OF SIX (6) INCHES.
7. THE COMPACTION EQUIPMENT TO BE USED FOR PROOF ROLLING THE FOOTPRINT SUBGRADE SHOULD CONSIST OF A SMOOTH DRUM VIBRATORY ROLLER HAVING A TOTAL STATIC WEIGHT OF AT LEAST 10,000 POUNDS. THE VIBRATORY ROLLER SHOULD BE OPERATED IN A FREQUENCY RANGE OF 100 TO 1300 VIBRATIONS PER MINUTE (V.P.M.). AT 1300 V.P.M. THE DYNAMIC FORCE SHOULD BE AT LEAST 20,000 POUNDS. THE TOTAL APPLIED FORCE (STATIC WEIGHT PLUS DYNAMIC FORCE) SHOULD BE AT LEAST 30,000 POUNDS.
8. ALL FOUNDATION WORK, INCLUDING BUT NOT LIMITED TO PROOF ROLLING, FOOTING PREPARATION AND FILL/BACKFILL PLACEMENT, SHOULD BE MONITORED AND TESTED BY A QUALIFIED REPRESENTATIVE OF PROFESSIONAL GEOTECHNICAL ENGINEERING FIRM TO ENSURE THE SPECIFIED BEARING CAPACITY IS AVAILABLE AT THE FOOTING SUBGRADE ELEVATIONS AND THE SPECIFIED DEGREE OF COMPACTION IS OBTAINED ON A LAYER BY LAYER BASIS.
9. IN THE EVENT DESIGN CONSIDERATIONS CHANGE OR UNUSUAL CONDITIONS ARE DISCOVERED DURING CONSTRUCTION, A LICENSED PROFESSIONAL GEOLOGIST SHOULD BE NOTIFIED IMMEDIATELY SO ADDITIONAL AND/OR REVISED RECOMMENDATIONS CAN BE DEVELOPED AS REQUIRED.

1. STEEL SUPPLIER SPECIFICATIONS SHALL OVERRIDE THE FOLLOWING NOTES BELOW WHEN THEY ARE MORE STRINGENT.
2. STRUCTURAL STEEL SHALL BE NEW STEEL CONFORMING TO THE FOLLOWING:  
  
W-SHAPE BEAMS . . . . . ASTM A992 (Fy = 50 KSI)  
HSS COLUMNS . . . . . ASTM A500 GRADE B (Fy = 46 KSI)  
L-SHAPE ANGLE UNTELS. . . . . ASTM A36 (Fy = 36 KSI)  
CONVENTIONAL BOLTS. . . . . ASTM A325  
BASE PLATES . . . . . ASTM A36 (Fy = 36 KSI)  
ANCHOR RODS. . . . . ASTM F1554 GRADE 36 (Fy = 36KSI)
3. ANCHOR BOLTS, LEVELING PLATES, OR BEARING PLATES SHALL BE LOCATED AND BUILT INTO CONNECTING WORK, PRESET BY TEMPLATES OR SIMILAR METHODS. PLATES SHALL BE SET IN FULL BEDS OF NON-SHRINK GROUT.
4. WELDED CONNECTIONS SHALL BE MADE BY APPROVED CERTIFIED WELDERS USING FILLER METAL CONFORMING TO E70XX OR F7X-EXXX WITH LOW HYDROGEN.
5. FIELD CUTTING OF STRUCTURAL STEEL OR ANY FIELD MODIFICATIONS OF STRUCTURAL STEEL SHALL NOT BE MADE WITHOUT PRIOR WRITTEN APPROVAL BY DESIGN PROFESSIONAL FOR EACH SPECIFIC CASE.
6. STRUCTURAL STEEL MEMBERS AND CONNECTIONS EXPOSED TO THE WEATHER SHALL BE PRIMED AND PAINTED.
7. STRUCTURAL STEEL FRAMING SHALL BE DESIGNED AND CERTIFIED BY A PROFESSIONAL ENGINEER. CONTRACTOR SHALL CORRESPOND WITH REPRESENTATIVES OF STEEL MANUFACTURING. DESIGN REACTIONS SHALL BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO ORDERING.
8. FOUNDATION ELEMENTS WERE DESIGNED BASED ON DESIGN REACTIONS PROVIDED BY STEEL COLUMN MANUFACTURER, FINAL DESIGN REACTIONS OR ANY REVISED CALCULATIONS DRAWINGS, ETC... SHOULD BE PROVIDED TO OUR OFFICE FOR EVALUATION.

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